APPENDIX A ANNEX G-4

G-4: CEPP PIR Flow Easement Basin (FEB) Preliminary Seepage Analysis

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Geotechnical Seepage Analysis CEPP Flow Easement Basin (FEB)

- 1. General. This section is a description of the geotechnical seepage analysis performed for the Flow Equalization Basin (FEB) feature associated with the Central Everglades Protection Project (CEPP). The FEB is situated in South Florida and lies south of the Bolles Canal, north of Levee L-5, east of the Miami Canal (L-23) and west of the EAA project. It is to be bordered to the east by the STA3/4 Inflow Canal and to the South by the STA3/4 Supply Canal. The proposed FEB footprint is indicated in Figure G-1.
- 2. Regional Geology. The regional geologic conditions of the proposed FEB are varied in this area (see Figure G-1). Along the southern boundary near L-5, and the Miami Canal to the west, recent fill materials consisting of poorly graded gravel, sand, silt, and minor amounts of shells vary in thickness from 0.5 foot to 6 feet thick. This fill material extends to other canal areas of Broward County. Below this fill material and throughout most of the surficial cover of northern Broward County, Quaternary shelly-sediments of the Plio-Pleistocene age prevail. The shelly sediments consist of mollusk bearing sediments (sands and carbonates) (P. Schweitzer, 2010). These conditions may also be marginally applicable to the FEB site in Palm Beach County as well.

Below these sediments is the Miami Limestone (Pleistocene) which represents the upper portion of the well-known Biscayne Aquifer. Thickness of the Biscayne Aquifer varies considerably from 10 feet in the northwest end of the county to 240 feet by Ft. Lauderdale which is situated in eastern Broward County (U.S. Geological Survey, 1970); however, the Miami Limestone's maximum thickness is 40 feet toward the coast (U.S. Army of Corps of Engineers, 2011). The Miami Limestone consists of two facies (P. Schweitzer, 2010): an oolitic facies of white to orangish gray, poorly to moderately indurated, sandy, oolitic limestone with scattered concentrations of fossils and a bryozoan facies of white to orangish gray, poorly to well-indurated, sandy, fossiliferous limestone.

The Ft. Thompson Formation represents the base of the Biscayne Aquifer. It is over 200 feet thick in eastern Palm Beach County. This unit consists of alternating beds of marine, brackish and freshwater limestones. The hydraulic conductivity of this formation averages 40,000 feet per day (U.S. Army Corps of Engineers, 2011).

Groundwater within the Biscayne Aquifer is at or near the ground surface and is generally undulating conforming to the topography. The water table commonly slopes eastward toward the coast, except in the Everglades, where it slopes southward. The groundwater table normally lies within the Miami Limestone or organic soils of recent age. Water table fluctuations are heavily influenced by seasonal rainfall, natural discharge, and pumping.

3. Local Geology. The local geology of the FEB feature begins at the surface with peaty material (peaty clay and/or organic sand), generally with an average thickness of 5-8 feet. Below that are undifferentiated layers of sandy, clayey materials of marginal thickness. Below these are consolidated and unconsolidated sediments of the Ft. Thompson formation. The Ft. Thompson limestone is pitted to vuggy with quartz sand-filled voids. The thickness of the Ft. Thompson limestone in this area is indeterminable at this time due to sparse subsurface exploration data.

Underlying the Ft. Thompson formation is the Caloosahatchee Marl which consists of shelly sands and shell marls. Below this formation is the Tamiami Formation which has a maximum depth of 65 feet to the top of the formation. This is the base of the unconfined water-table aquifer (Schroeder et al, 1954). Within the FEB, the majority of the surficial soils are Histosols which includes Everglades peats and Loxahatchee peats (Gleason et al., 1974; Bruland, 2006). According to Gleason et al, 1974, Everglades peats develop on topographic high areas and are comprised of *Cladium* tissue. Everglades peats are typically brown to black with minimal mineral content. Loxahatchee peats are found in topographic low areas and are composed of the remains of the roots and rhizomes of *Nymphea*, a white water lily. Loxahatchee peats have been classified as the Terra Ceia series (Euic, hyperthermic Typic Haplosaprists) (Soil Conservation Service, 1978). The western margin of WCA-3A is mixed marl peats that are derived from the underlying limestone (Brown et al., 1991).

4. Seismicity. South Florida is considered to be one of the most seismically stable locations in the United States (Petersen, Mark D. et. al, 2008). Historically, only minor shocks have occurred, with only one that resulted in damage. Additional shocks of suspect origin have been recorded that were felt in the Everglades area. The three Florida shocks of doubtful seismic origin rumbled through the Everglades, La Belle/Fort Myers area in July 1930, Tampa in December 1940, and the Miami/Everglades/Fort Myers area in January 1942. Most authorities attributed these incidents to blasting, but a few contend that they were seismic.

ER 1110-2-1806, (1995) indicates that South Florida is in Seismic Zone 0 (areas with least potential for seismic activity). Since no capable faults or recent earthquake epicenters are known to exist near the project site, the possibility of accelerations at the site approaching that required to induce liquefaction of the subsurface is remote. However, since this is a planned permanent impoundment and the underlying soils contain loose sand and silt granular material, a liquefaction screening evaluation will be conducted in accordance with CERP Design Criteria Memorandum No. 6. This screening will be conducted once the geotechnical investigations are completed.

5. Existing Project Data. There is only a sparse amount of subsurface geotechnical data available for the FEB. Existing subsurface used to develop the eastern dike seepage analysis cross section was taken from the EAA Reservoir A-1 Geotechnical Data Report of March 2006. A core boring and recharge test was also taken within the reservoir area with the core boring log CP02-EAARS-CB-0002 presented in Appendix G-1. This information was obtained from Report No. 02-042, Ardaman and Associates, 2003. To evaluate the seepage on the west side of the FEB, C & SF Part I Agricultural and Conservation Areas, Supplement 1 – Geology and Soils, December, 1951 provided geologic cross sections that provide subsurface information for the north and west sides of the proposed FEB impoundment. The western boundary subsurface profile can be approximately defined by Figure G-2 along L-23 from sta. 0+00 to sta. 600+00. The northern subsurface boundary of the FEB can be approximated by the cross sections taken along L -22 shown on Figures G-3 and G-4 from sta. 532+18 to sta. 0+00.

6. Seepage Model. The seepage analysis was performed on two sections of the proposed perimeter levee and canal system. The models consist of one typical section for the east dike and one of the west dike. A copy of the idealized geometry and water surface elevations used in the seepage analysis are shown on Figures G-5 and G-6. The computational tool used to model the seepage for the east and west cross sections was the two dimensional finite element program SEEP/W. The east and west SEEP/W representations of the input model are shown on Figures G-7 and G-8. Layers and hydraulic input parameters used in the SEEP/W cross sections are provided in Tables G-1 and G-2. The impoundment water level was kept at el. 10.0 ft with tailwater at the dike toe at elevation 6.0 ft.

The subsurface characterization for the model was conducted using an idealized geologic profile utilized for the EAA project immediately east of the project feature site. Due to the extremely limited geotechnical exploration data in the area of the FEB footprint, the modeled results presented provide only a tentative estimate of seepage quantities. The western cross section model was based on a recharge test at location RT-2 near core boring CP02-EAARS-CB-0002. This boring, which is located in the middle of the proposed reservoir, was performed in the 2003 geotechnical exploration program for the EAA and as described by Report No. 02-042, Ardaman and Associates, 2003. Other subsurface information used for development of the west section model was the C & SF Part I Agricultural and Conservation Areas, Supplement 1 – Geology and Soils, December, 1951 and C & SF Part I Agricultural and Conservation Areas, Supplement 7 – Permeability Investigations by Well Pumping Tests, February, 1953, and Report of Investigations No. 13 (RI-13), Water Resources of Palm Beach County, Florida, 1954.

Table G-1. Eastern Seepage Model Cross Section Material Properties

Layer	Elevation Extent (feet)	Kx, Horizontal Hydraulic Conductivity	Anisotropy Ratio, Ky/Kx
		(fpd)	
Dike Sand	EL. 15.0->EL. 6.0	3.0	1.0
Caprock Limestone	EL. 6.0->EL.3.0	100	0.1
Upper Okeechobee-	EL. 3.0-> -13.0	60	0.42
Upper Limestone			
Upper Okeechobee-	EL13.0 ->-24.0	200	0.375
Lower Limestone			
Lower Okeechobee-	EL24.0 ->-56.2	250	0.5
Upper Sands			
Tamiami Formation	EL56.20 ->-89.9	36	0.5

Table G-2. Western Seepage Model Cross Section Material Properties

Layer	Elevation Extent (feet)	Kx, Horizontal Hydraulic Conductivity	Anisotropy Ratio, Ky/Kx
		(fpd)	,,
Dike Sand	EL. 15.0->EL. 6.0	3.0	1.0
Caprock Limestone	EL. 6.0->EL.3.0	283	0.1
Upper Okeechobee-	EL. 3.0-> -13.0	72	0.42
Upper Limestone			
Upper Okeechobee-	EL13.0 ->-24.0	200	0.375
Lower Limestone			
Lower Okeechobee-	EL24.0 ->-56.2	250	0.5
Upper Sands			
Tamiami Formation	EL56.20 ->-89.9	36	0.5

The regional hydrogeologic features of the project feature area show the surficial aquifer of sand, shell and limestone tends to thicken from the western boundary at Hendry County to the eastern edge of Palm Beach County. However, from north to south, the surficial aquifer top limestone beds thicken somewhat from Lake Okeechobee to about five miles south where they become more uniform in thickness. Supplement 1 (1951) indicates the presence of cavities in the top layers of the limestone in selected borings from 1-3 feet in thickness. These, however, do not seem to be continuous, at least along the line of borings. These features may have a profound effect on the hydraulic conductivity of the subsurface strata on the west side of the reservoir. To compensate for this, the hydraulic conductivities for the east section model were increased by 20% to account for a roughly 5.2% increase in porosity over the strata matrix hydraulic conductivity values. The caprock permeability in the west section model was increased from 100 ft/day to 283 ft/day based on results of the recharge test of Report No. 02-042 (2003).

The resulting seepage quantity for the east section was 320 cubic feet/day/ft of levee, and for the west section the seepage quantity was 387 cubic feet/day/ft of levee.

7. Future Geotechnical Investigations. The seepage quantities presented for the FEB are tentative estimates. As they are a basis for the project estimate, the subsurface materials need to be characterized by a geotechnical exploration and laboratory testing program. There is a very real possibility that previous core borings did not pick up key flow channels within the underlying limerock layers and these could be so conducive to flow that they will control the seepage losses for the reservoir. In addition, laminar flow assumptions used in the Darcy's Law based models may not be applicable in some areas. Scaled test sections at the adjacent EAA project show that maintaining an operating reservoir head level over an extended period of time may be difficult without seepage control systems such as cutoff walls or reservoir bottom treatment. Furthermore, there is a potential for piping of embankment material if there are cavities and natural pipes in the levee foundation. A comprehensive geotechnical exploration program featuring components such as reservoir foundation clearing and mapping along with geophysical testing, full-scale pump testing

along with design stage core boring exploration and geotechnical laboratory testing will be required. It is recommended to begin planning for such investigations as expeditiously as possible.

8. Borrow Materials. Sands from the proposed existing borrow from the exterior canal can be used as dike fill material. Whether they can be used without processing of limestone rock fragments is to be determined. With removal of organic surficial soils, dike foundation bearing and tolerable settlement levels are anticipated.

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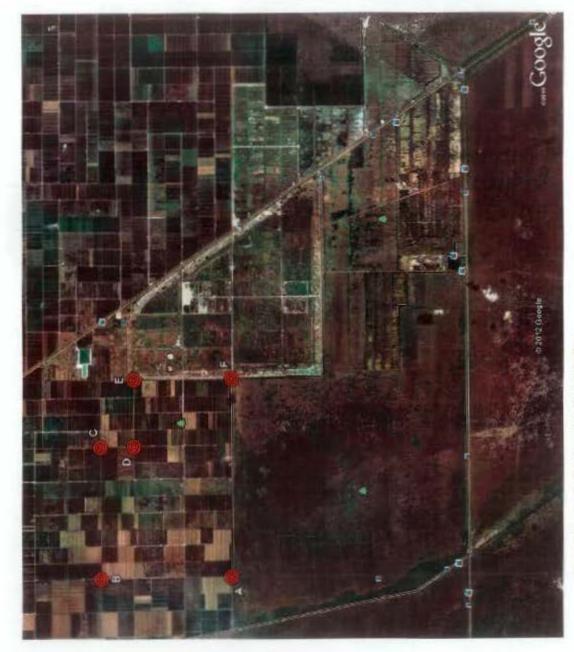


Figure G-1. Aerial Photograph of Project Site

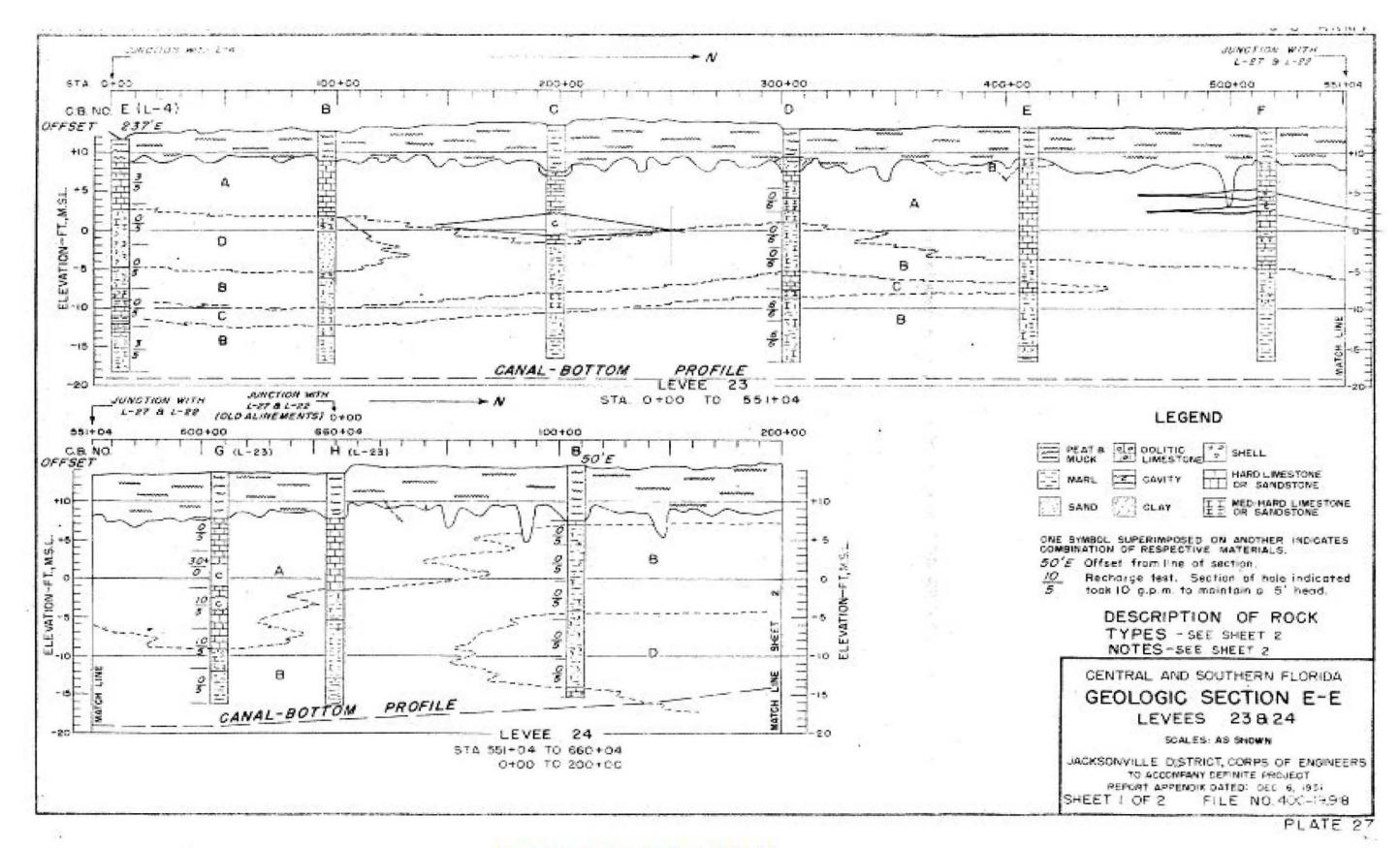


Figure G-2. Geologic Section L-23 & L-24

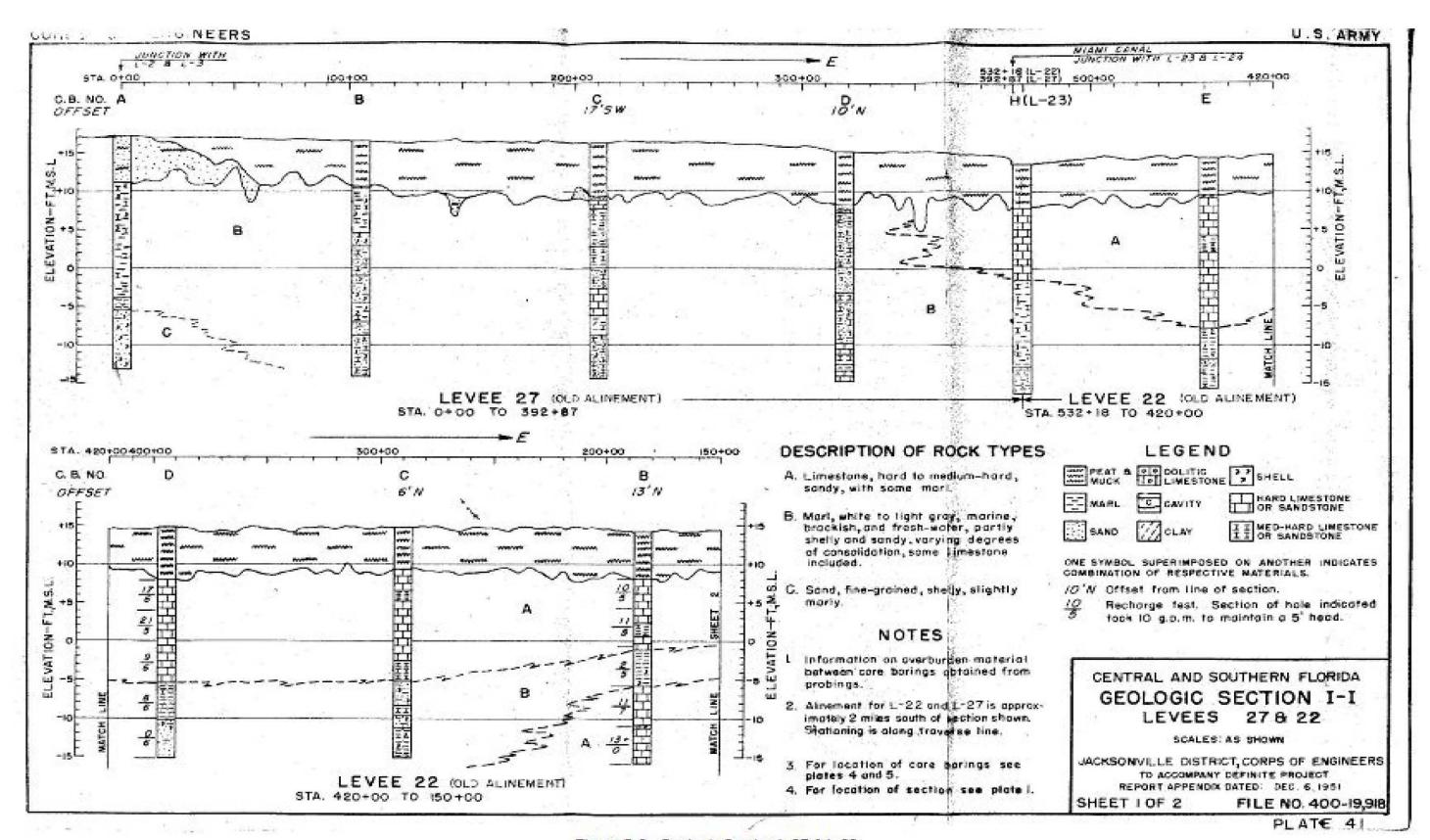


Figure G-3. Geologic Section L-27 & L-22

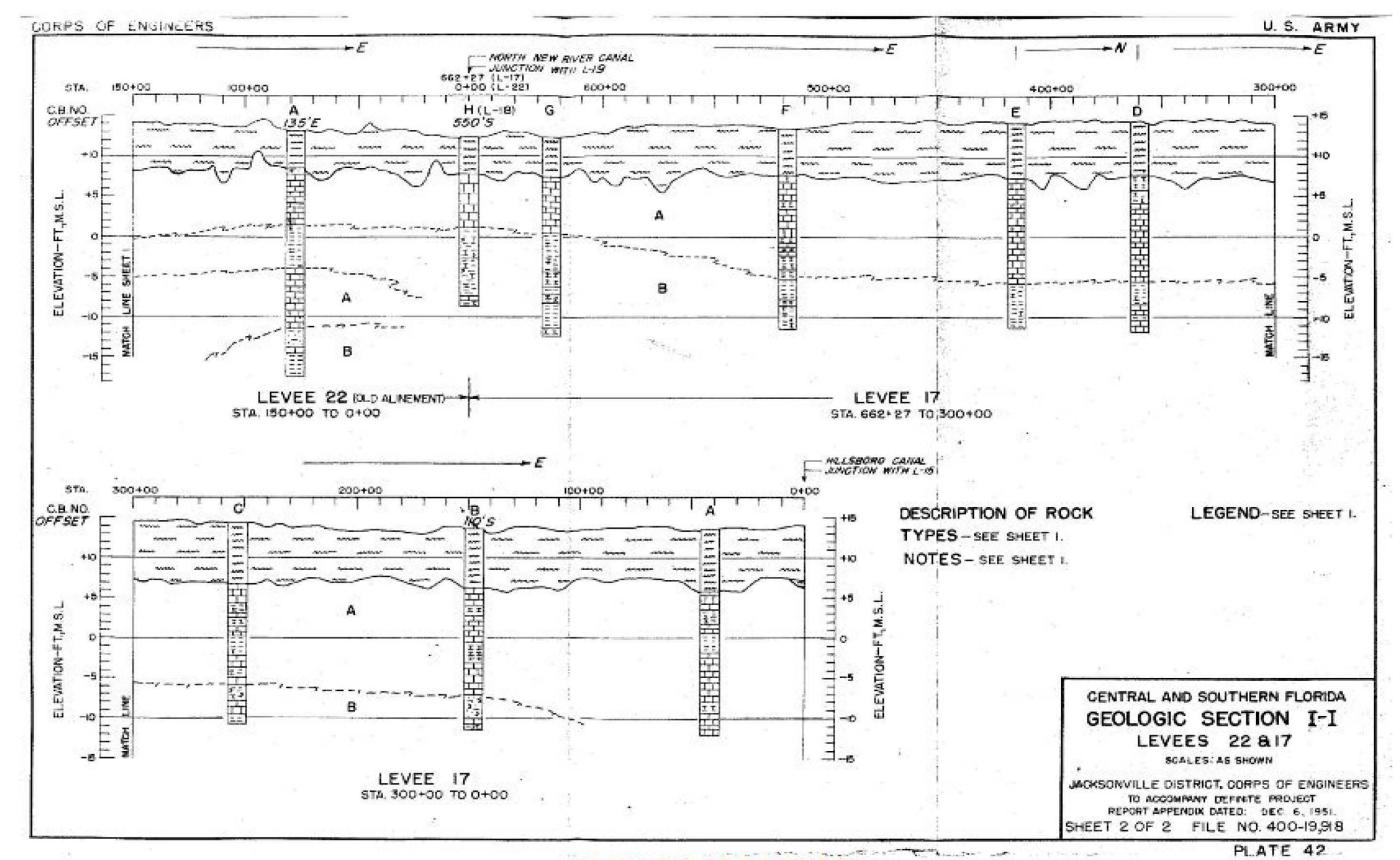
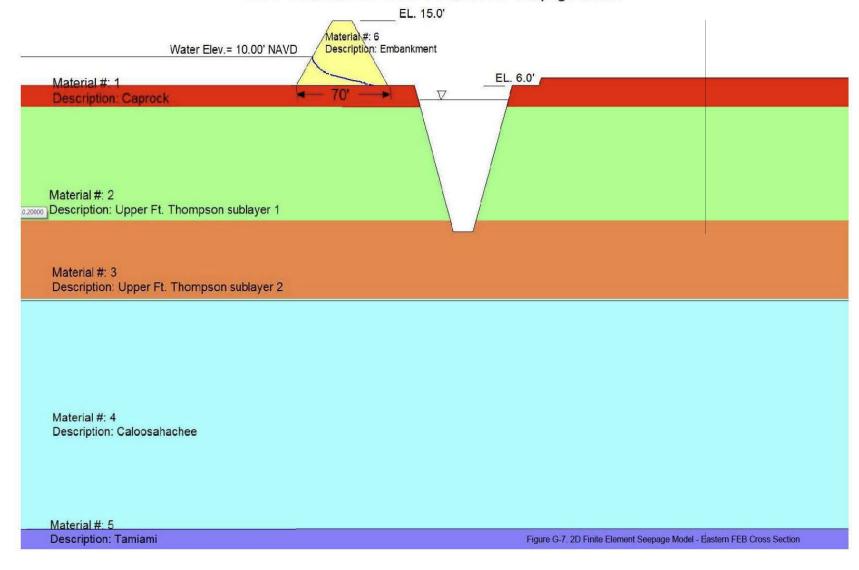


Figure G-4. Geologic Section L-22 & L-17

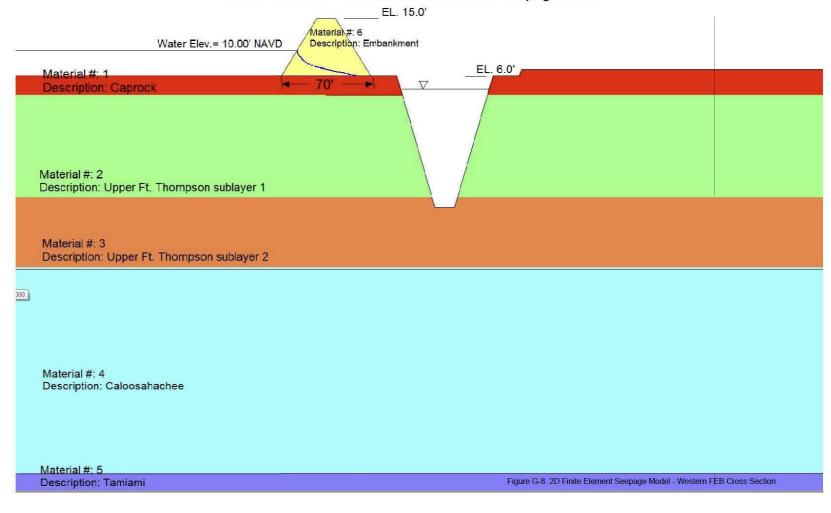
Figure G-5. FEB Seepage Model Input Data

Figure G-6. FEB Seepage Model Idealized Cross Section Geometry

CEPP Eastern FEB Cross Section FEB Seepage Model



CEPP Western FEB Cross Section FEB Seepage Model



APPENDIX G

CORE BORING

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t								-42.0			17	
E					87	36			SPT Sam	pler —	20	1
	RM 18	1							(Continu		9.0	- 5

_			G (Cont. Sheet)	-	ksonville	-	_		*	OF 10 S	HEET
PROJEC			relatives the section	1203930	DINATE S			UM	HORIZONTAL	VERTICAL	
-	ON COORD		gricultural Area Reservoirs	_	te Plane	-	_	-	NAD83	NAVD88	_
			75,528	0.00000	O.FI.	. 0	OHIN	•			
ELEV.	DEPTH	CEGEND	GLASSIFICATION OF MATE	RIALS	nec.	POX OR	800 08 08		REMARKS	BLOWS.	NVALUE
		100			87	36		-43.6	SPT Sample	er 19	39
- 3	-	100					1			15	Т
	F	12.			87	37			SPT Sample	er 19	1
45.0	57.0	100						-45.0		22	41
	-	1.00	SAND, poorly-graded with clay,					-40.0		12	1
			medium-grained sand, little clay, angular shell, trace fine gravel-s		100	38			SPT Sample		1
			gray (SP-SC)	EGG SHOW	100				Of F Dalique	13	29
1 3		1 6	110000000000000000000000000000000000000		-	-	1	-46.6			+
3		1.18	At El. 47.0 Ft., few shell, trace of	var					Par 6	14	1
- 3		1 2	7.9.00 77.00 07.00 07.00 0.00	-01	60	39	ш		SPT Sample		36
		1.18				_		-48.0		19	-
		1.00					Н			10	1
		1.18			93	40			SPT Sample	er 14	28
-49.6	61.5	LB						-49.6		14	- 40
			SAND, clayey, mostly fine to medium-grained sand, some cla	v little fine				7 3000		10	
		88	gravel-sized shell, gray (SC)	J. mine inne	87	41	П		SPT Sample	er 19	١
		323					ш	-51.0		31	50
	6						1 1			10	
- 1					93	42			SPT Sample	er 13	1
1	-	200			1000		ı	#2 A	32.3.3307	12	25
			At El52.6 Ft., some shell, trace	day, lens				-52.6		13	+
1		223	of clay		73	43			SPT Sample	The second second	+
1	8				1.0				or i gampi		33
- 1	-	32			\vdash	-		-54.0		20	-
- 1	5 1	92				122	ш		0222-25719	10	4
		223			100	44			SPT Sample	(miles and other states)	22
-55.6	67.5	200	2115					-55.6		11	
- 1		1:33	SAND, poorly-graded with clay, r (SP-SC)	mostly shel						10	
t		- 2			67	45			SPT Sample	er 12	28
Ł								-57,0		16	20
1	/:	. 8								16	
F					93	46			SPT Sample	er 14	1
F								-58.6		12	26
1		. 8								11	
1		: 2			53	47			SPT Sample	-	1
t		-33			1	105.00		60.0	STATE STATE OF THE PARTY	16	28
t	-	18						-60.0		18	-
£		: 8			49	40			PDT 0	- American	1
F	2]	. 8			67	48			SPT Sample	-	33
ŧ		13	At El61.6 Ft., mostly shell					-61.6		19	
1	8 3	1	PA CIL -O LO FY. MOSEY SCIEN							- 6	
t		. 2			87	49			SPT Sample	12	27
- 1		.20			1 1			-63.0		15	185

DR	ILLING	LO	G (Cont. Sheet)	Jacks	onville	Dist	rict				OF 10 S	HEETS
PROJEC				COORDIN				UM	HORIZONTAL		RTICAL	
_		_	gricultural Area Reservoirs	State		_	_	_	NAD83	1 1	NAVD88	_
	ON COORS 736,775			12.0 F		OF I	HINO	G				
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MATER	200	REC.	BOX OR SAMPLE	ROD OR UD		REMAI	tes	BLOWS 0.5 FT.	N-VALUE
	-	1.18			Т						20	
-63.8	75.7		Limestone, fine-grained, trace of o	lay trace	93	50	Ш		SPT Sar	mpler	18	1
	F	垚	of phosphate, gray	100000				-64.6			18	36
à	-	去									14	
		窑			53	51			SPT Sar	mpler	16	33
-66.0	78.0	磨						-66.0			17	33
	1		SAND, poorly-graded, mostly fine medium-grained quartz, trace san	to dstone.							10	
3	-	100	trace shell, fight gray (SP)		67	52			SPT Sar	mpler	40	85
3						_		-67.6			45	
	_										26	
					67	53			SPT Sar	mpler	32	81
	-	100						-69.0			49	
3					5.5				4.44		36	-
	-				100	54			SPT Sar	npler	65	122
		333			\vdash	-		-70.6		_	57	-
	-								007.0		10	1
- 1					87	55			SPT Sar	npier	20	34
	-					_		-72.0		_	14	-
					93	56			SPT Sar	molar	28	-
	-				35	50			SF1 S#	ripres	29	57
					\vdash			-73.6			14	-
					53	57			SPT Sar	moler	17	1
					-			-75.0				
								-73.0			8	
1					87	59			SPT Sar	npler	18	
						200		-76.6	15Milles	100000	29	47
		100						-10.0			24	
1					87	60			SPT Sar	mpler	34	1_
1								-78.0			42	76
1											9	
-					73	61			SPT San	npler	18	200
ł		100						-79.6			20	38
ł			From El79.6 to -80.6 Ft., mostly coarse-grained quartz, trace phose	ohate.							4	
-80.6	92.5		trace shell, trace sandstone, light of		93	62			SPT San	npler	10	16
			SAND, clayey, mostly medium to coarse-grained sand, some clay, to	race				-81.0			6	10
t		XX	phosphate, trace shell, gray (SC)								_ 5	
ŀ					100	63		la 10 - 10 -	SPT San	npler	7	19
t			At El82 0 Ft., trace sand					-82.6			12	100
AJFO		1992	AI El82.6 Ft., little shell, little lime	estone	100	64			SPT San	npler	3	

DR	ILLING	LOC	3 (Cont. Sheet)	Jacks	onville	Dist	ict			SHERT 6	EFTS
PROJEC	т			COORDIN	_	-	-	UM	HORIZONTAL	VERTICAL	
CER	P Evergla	des Ag	gricultural Area Reservoirs	State	Plane,	FLE	20120	1000	NAD83	NAVD88	
	ON COORD			ELEVATE		OF 8	ORIN	G		over the state of	
X = 7	736,775	Y = 7	75,528	12.0 8	1	_	_				_
ELEV.	DEPTH	CEGEND	CLASSIFICATION OF MATERIA	ALS	nic.	BOX OR	MOD OR UD		REMARKS	BLOWN.	M-VALUE
					100	64		-84.0	SPT Sampl	er 9 7	16
			At El84.0 Ft., trace sand							_10	
					100	65		-85.6	SPT Sample	er 8 9	17
							1	-		18	
			At El86.0 Ft., trace limestone		87	66		97.0	SPT Sample	er 21 26	47
			At El87.0 Ft., some shell					-87.0		12	\vdash
					67	67			SPT Sampl	er 22	51
-88.6	100.5		SAND, clayey, mostly fine to coarse	-nrainet	-			-88.6		29	
			sand, some clay, few shell, trace pt gray (SC)	nosphate.	100	68			SPT Sample	14 er 9	23
-90.0	102.0							-90.0		14	23
		鑘	Sandstone, fine-grained, few shell, clay, trace of phosphate, gray	trace of					7950 Com (c) 40	5	
		羉	and the state of t		67	69			SPT Sample		36
- 1					-	_		-91.6		20	-
		圞			100	1			2022	17	-
					100	70		-93.0	SPT Sample	er 16	33
-					\vdash	1		193.0		9	-
1					87	71			SPT Sample	er 17	
- 1						200		-94.6		19	36
- 1		盟			\vdash			-94.0		3	
- 1					67	72			SPT Sample	-	
- 1								-96.0	9503955963	30	49
- 1								-80.0		19	
1					73	73			SPT Sample		100
1								-97.6	0.70.20.80.0000	23	42
ţ	1	羅						-57.50		24	
ţ		翻	At El -98.0 Ft., few day		93	74			SPT Sample		1
F								-99.0		22	43
F										22	
F					93	75			SPT Sample	er 24	43
F								-100.6		19	43
t		2								_ 18	
t					93	76			SPT Sample	N 15	35
£		5						-102.0		20	-
t		177			87	77			SPT Sample	14	
AJFO					-	4.4			Or I distribu	16	

DRI	LLING	roo le	Cont. Sheet)	Jackso	nville	Distr	ict				SHEETS	4
PROJEC			MILL DE PROPERTY DE L'ANDRE DE L'	COORDINA			DAT	UM	HORIZONTAL	VERTICAL		1
			flural Area Reservoirs	State F		_			NAD83	NAVD88		4
	OH COORD		-	BLEVATIO		OF 8	ORIN	G				
X×7	736,775		28	12.0 F		-				-		4
BLEV.	DEPTH	LEGGING	CLASSIFICATION OF MATERIA	ALS.	nèc.	SAMPLE	RGB OR UB		REMARKS	6604		
		100			87	77		-103.6	SPT Samp	er 17	33	1
										15		1
					87	78			SPT Samp	er 17	- 31	
	-				\vdash			-105.0		12	-	1
						_			ORT Com	_	_	ł
		200			93	79			SPT Samp	_	21	I
		E COL			\vdash	_		-106.6		- 1	_	4
										_ 1	_	١
					100	80		1	SPT Samp	-	- 33	П
		339						-108.0		- 16	5	4
										12	2	1
		200			93	81			SPT Samp	ler 1	32	
	3							-109.6		11	3	
		羅維						-		1	7	ł
					87	82		l	SPT Samp	ler 2	39	J
		200						-111.0		1	7	
	-	200					1	-		1	2	7
		CO. T.			93	83			SPT Samp	fer 1	3	. 1
	-	BEX						-112.6		1	32	1
- 3		24.00				\vdash	1	-11E-0		1	6	1
	-	27			73	84		1	SPT Samp	_	7	-
					1.0	04	1		and the second	1	41	ч
	-	C.			\vdash	\vdash	1	-114.0		1	_	4
		節座						1	SPT Samp	_	-	
		EACH.			87	85			ar i aimp		3 27	1
		100			\vdash	⊢	1	-115.6			_	-
		100			6.5			1		_	4	-
		1			93	86	1		SPT Samp	_	- 26	8
10					\vdash	┖	1	-117.0		1	_	4
		100								1	-	1
		100			73	87	1		SPT Samp	-	4 24	٠
							1	-118.6			0	
		200						1		-1		
		1000			67	88	1		SPT Samp	ler 1	0 25	اء
	-	200						-120.0		1	5	
	-	1000					1			2	6	
		255			80	89	1		SPT Samp	ier 1	5	. 1
		201						-121.6		1	6 3	1
- 1		王等				\vdash	1	100		2	3	٦
		123			87	90	1	1	SPT Samp		4	
		1			1	1	1	¥92.5		-	8 43	2
	ORM 18	PORTO:			_	_	_	-123.0	(Continu			_

Boring Designation CP02-EAARS-CB-0002 **DRILLING LOG (Cont. Sheet)** OF 10 SHEETS Jacksonville District PROJECT COORDINATE SYSTEM DATUM HORIZONTAL VERTICAL State Plane, FLE CERP Everglades Agricultural Area Reservoirs NAD83 NAVD88 LOCATION COORDINATES ELEVATION TOP OF BORING X = 736,775 Y = 775,528 12.0 Ft. SAMPLE COS CASO N-VALUE CLASSIFICATION OF MATERIALS nec. REMARKS 9 10 60 91 SPT Sampler 21 11 124.6 17 92 93 SPT Sampler 24 43 19 -126.0 25 28 80 93 SPT Sampler 48 20 -127.6 14 140 73 94 SPT Sampler 18 37 19 -129.0 12 95 87 SPT Sampler 16 55 39 -130.6 31 131.0 143.0 SAND, poorly-graded, mostly fine to medium-grained quartz, trace phosphate, light gray (SP) SPT Sampler 100 96 37 61 24 132.0 17 97 21 100 SPT Sampler 38 17 133.6 14 SPT Sampler 100 16 36 20 -135.0 12 100 99 25 SPT Sampler 52 27 -136.6 12 100 100 SPT Sampler 18 36 18 -138.0 11 93 101 SPT Sampler 16 34 18 -139.6 10 SPT Sampler 100 102 16 38 22 141.0 15 67 103 SPT Sampler 26 51 25 142.6 154.5 -142.6 Sandstone, fine-grained, some quartz sand, 80 104 SPT Sampler 18

G-8

(Continued)

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SAJ FORM 1836-A JUN 02

PROJE			G (Cont. Sheet)	COORDIN	_	Distr	_	TIM	HORIZONTAL	OF 10 SI	LETS	+
		ades A	gricultural Area Reservoirs	State			MIDAI		NAD83	NAVD88		L
	ION COOR		A CONTRACTOR OF THE PARTY OF TH	ELEVATIO	and the same in case of	-	ORIN	G	10000	1 1011000		1
X =	736,775	Y = 7	775,528	12.0 F	1.		A. A.Y					1
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MATER		nic.	BOX OR SAMPLE	MOD OR UD		REMARK	a grows	NVALUE	
			few day, trace shell, trace phosph At EL -143.0 Ft., few shell	ate, gray	80	104		144.0	SPT Same	pler	39	Ī
					93	105			SPT Samp		48	E
	E		At Et145.6 Ft., trace clay					-145.6		9	F	ŧ
					100	106		-147.0	SPT Samp	oler 17 41	58	ŧ
					73	107		-148.5	SPT Same	16 20 20	40	-
	-				80	108		-150.0	SPT Samp	15 17 19	36	-
					80	109		-151.6	SPT Samp	30	38	F
					73	110			SPT Samp	18	37	I
					80	111		-153.0	SPT Samp	14 oler 16	34	-
	-				80	112		-154.6	SPT Samp	-	37	-
					73	113		-156.0	SPT Samp	-	31	-
					100	114		-157.6	SPT Samp	14 14 bler 14	30	E
159.0	171.0	1	SAND, poorly-graded, mostly fine t medium-grained quartz, trace fine gravel-sized sandstone, trace phos		100	***		-159.0	COT C	16 15	-	-
161.0	173.0		grave-sized sandssone, trace prior light gray (SP)	egil lyttell,	100	115		-160.6	SPT Samp	19 14	43	F
	-		Sandstone, medium-grained, some sand, few clay, few shell, trace of p gray	quartz phosphate.	100	116		-162.0	SPT Samp	-	28	E
AJ F					87	117			SPT Samp	ler 24		-

			G (Cont. Sheet)	Jackso	UAING	Uistr	ict			OF 10 SE	EETS
PROJE				COORDINA						TICAL	
		_	pricultural Area Reservoirs	State P	_			1 NAD83	; N	AVD88	_
	ON COORD			ELEVATIO		OF B	ORIN	G			
X =	736,775		75.528	12.0 Ft	_		_			-	
ELEV.	DEPTH	LEGEND	CLASSIFICATION OF MATERIA	us	RĒC.	BOX OR	OR UD	***	IARKS	BLOWS 0.5 FT.	M-VALUE
- Ş					87	117		-163.6 SPT	Sampler	18	42
					67	118		SPT:	Sampler	16 16	34
					\vdash		1	100.0		17	
					100	119		SPT	Sampler	18	1
	-	Щ			100			2000		19	37
		题			\vdash	-		-166.6		-	-
	-				400				Name of the last	25	1
	-				100	120			Sampler	26	55
168.0	180.0				_	_		-168.0		29	
			Soils are field visually classified accordance with the Unified Soils Classification System. Laboratory Testing Results SAMPLE SAMPLE LABORA ID DEPTH CLASSIFIED 119 177.0/178.5 * "Lab visual classification based on curve. No Atterberg limits. 3. Additional Laboratory Testing 119 Moisture Content.	ATORY				Abbreviations: NR = Not Recorded. DT = Drill Time. HP = Hydraulic Press	sure.		